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O'BRIEN AND GERE ENGINEERS INC PHILADELPHIA PA JUSTIN--ETC F/G 13/2  
NATIONAL DAM SAFETY PROGRAM. ROBERT G. STRUBLE DAM (NDI-PA 0062--ETC(U)  
JUL 79

DACW31-79-C-0010

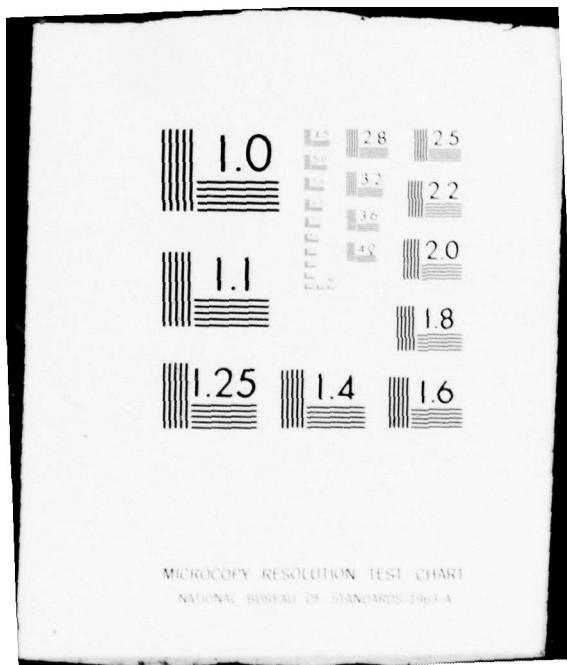
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MICROCOPY RESOLUTION TEST CHART  
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DELAWARE RIVER BASIN  
EAST BRANCH BRANDYWINE CREEK, CHESTER COUNTY

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PENNSYLVANIA

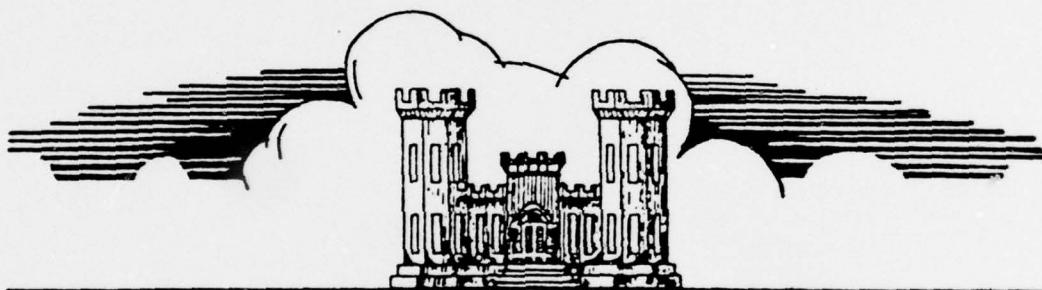
LEVEL A

## R. G. STRUBLE DAM

NDI - PA 00621  
PA DER 15-298  
SCS PA - 431



### PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM



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Prepared By  
**O'BRIEN & GERE**  
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DEPARTMENT OF THE ARMY  
BALTIMORE DISTRICT CORPS OF ENGINEERS  
BALTIMORE, MARYLAND

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DELAWARE RIVER BASIN

Name of Dam: Robert G. Struble Dam

County and State: Chester County, Pennsylvania

Inventory Number: PA 00621

PHASE I INSPECTION REPORT

NATIONAL DAM SAFETY PROGRAM

(6) National Dam Safety Program, Robert G.  
Struble Dam (NDI-PA 00621, PA DER 15-298,  
SCS PA-431), Delaware River Basin, East  
Branch Brandywine Creek, Chester County,  
Pennsylvania. Phase I Inspection Report.



(11) Jul 79

Prepared By:  
O'BRIEN & GERE ENGINEERS, INC.  
JUSTIN & COURTNEY DIVISION

(13) DACW31-79-C-0019

For:  
DEPARTMENT OF THE ARMY  
Baltimore District, Corps of Engineers  
Baltimore, MD. 21203

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## PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigations, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected, and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the spillway design flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. The spillway design flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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PHASE I REPORT

NATIONAL DAM INSPECTION PROGRAM

Name of Dam: Robert G. Struble Dam ID # PA 00621  
State Located: Pennsylvania  
County Located: Chester County  
Stream: East Branch Brandywine Creek  
Coordinates: Latitude 40° 6.7' Longitude 75° 51.9'  
Date of Inspection: May 2, 1979

ASSESSMENT

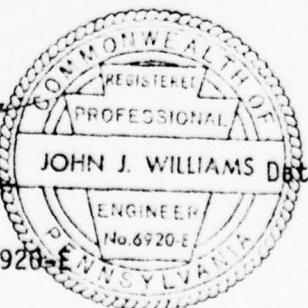
Robert G. Struble Dam is an earth embankment with a drop inlet closed conduit (principal) spillway and a vegetated earth cut (emergency) spillway. The embankment is approximately 1,500 feet long with a maximum height of 31 feet and impounds a reservoir with a normal pool storage capacity of 1,025 acre-feet. The dam is located on the East Branch of Brandywine Creek about 3 miles Northeast of Honeybrook, Pennsylvania.

The Spillway Design Flood (SDF) for this "Intermediate" size, "High" hazard dam is the Probable Maximum Flood (PMF). The combined spillway system is capable of discharging 100 percent of the PMF without overtopping of the earth embankment. Therefore the spillway is considered "Adequate".

Based on the visual observations and review of the information obtained from the Pennsylvania Department of Environmental Resources, Division of Dam Safety, Struble Dam is considered to be in good condition. No further investigations or remedial measures are recommended at the present time.

O'BRIEN & GERE ENGINEERS, INC.  
JUSTIN & COURTNEY DIVISION

J. J. Williams,  
Vice President  
Pennsylvania Registration #PE 006920-E



Approved By

Date: 1 Aug 79



*R. G. STRUBLE DAM, CHESTER COUNTY, PENNSYLVANIA*  
OVERVIEW

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PHASE I REPORT  
NATIONAL DAM INSPECTION PROGRAM  
ROBERT G. STRUBLE DAM  
NDI I.D. PA 00621

SECTION I

PROJECT INFORMATION

1.1 General

- a. Authority. The Dam Inspection Act, Public Law 92-367, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a program of inspection of dams throughout the United States.
- b. Purpose. The purpose of this inspection is to determine if Struble Dam constitutes a hazard to human life or property.

1.2 Description of Project (Supplemented by information obtained from the Pennsylvania Department of Environmental Resources (DER), Division of Dam Safety, Harrisburg, Pennsylvania)

*POSTED*

- a. Dam and Appurtenances. Struble Dam is a zoned earth embankment, approximately 1,500 feet in length with a maximum height of 31 feet. The dam, a multiple purpose structure, was constructed as a portion of the Brandywine Creek Watershed Project. The reservoir has a normal pool storage capacity of 1,025 acre-feet for sediment accumulation and fish and wildlife. The normal pool surface area is 146 acres. An additional storage capacity of 596 acre-feet is utilized for floodwater control. The top of dam elevation is 625.5, the crest width is 14 feet, and the side slopes are 3 horizontal to 1 vertical (3H:1V) upstream and 2.5H:1V downstream. A 12-foot wide berm is provided on the upstream face at Elevation 612.8 and the slope is lined with 18-inch thick riprap from the berm to Elevation 618.8. According to the "As-Built" plans, the upstream and central portions of the embankment are composed of Zone I material, which is classified as a clayey silt. The central portion includes a cutoff trench with a 12-foot bottom width. The downstream shell (to Elevation 615.8) of Zone II material is classified as silty sand. The two zones are separated by a four-foot wide chimney drain which terminates in a drain trench at the downstream toe of the Zone I material.

*RESEARCH*

According to the "As-Built" plans, the principal spillway consists of a single stage reinforced concrete drop inlet structure, a 30-inch diameter prestressed concrete pipe and a reinforced concrete impact basin at the pipe outlet. The inlet structure, or riser, is situated approximately 40 feet upstream of the embankment axis with its crest (Elevation 615.8) directly above the upstream berm. A pond drain consisting of an inlet structure at the upstream toe of the embankment and 50 feet of 18-inch diameter prestressed concrete pipe outlets into the upstream wall of the riser. Pond drain discharge is controlled by a sluice gate mounted on the inside face of the upstream wall of the riser. Reinforced concrete anti-seep collars are provided for the pond drain and principal spillways pipes. A trapezoidal discharge channel with a 10-foot bottom width leads from the impact basin to a point 300 feet downstream of the dam where the discharge channel blends in with the natural channel. The initial 20-foot section of the outlet channel is riprap lined.

A 150-foot wide vegetated earth emergency spillway with 3H:1V side slopes is located in the left abutment area. A 240-foot long forebay channel is constructed upstream of the spillway crest on a 2% grade and a 300-foot long exit channel is constructed downstream from the crest on a 3.3% grade. The crest is a 30-foot long level section at Elevation 619.5. A vegetated earth dike is located on the right side of the emergency spillway to protect the embankment from any flow through the emergency spillway.

- b. Location. Struble Dam is located on the East Branch of Brandywine Creek, approximately 3 miles northeast of Honey Brook, Pennsylvania. The dam is situated within Honey Brook Township in Chester County. The dam site is shown on the USGS Quadrangle entitled "Wagontown, Pennsylvania" at coordinates N 40° 6.7', W 75° 51.9. A regional location plan of Struble Dam is enclosed as Plate 1, Appendix E.
- c. Size Classification. The dam has a maximum height of 31 feet and a maximum pool storage capacity of 2,880 acre -feet. Therefore, the structure is in the "Intermediate" size category.
- d. Hazard Classification. A dam failure could cause loss of lives and extensive property damage in the town of Cupola which is located about 1.5 miles downstream of the dam. Therefore, the dam is in the "High" hazard category.
- e. Ownership. Struble Dam is owned by the Chester County Water Resources Authority, 406 Farmers & Mechanics Building, West Chester, PA 19380.

- f. Purpose of Dam. The dam provides sediment, fish and wildlife storage at normal pool and floodwater storage at maximum pool (emergency spillway crest).
- g. Design and Construction History. Struble Dam was designed by the engineering staff of the Soil Conservation Service (SCS) of the United States Department of Agriculture and was constructed by Roger E. Gerhart, Inc. Construction began in May of 1971 and was completed in November of the same year. The dam was known as Supplee Dam until October 26, 1971, when it was officially dedicated as the Robert G. Struble Dam. There is no record of any subsequent modifications to the dam.
- h. Normal Operating Procedures. The only operating mechanism is the pond drain sluice gate which controls the pond drain. There is no written record of operation of the sluice gate; however, according to David Yaeck, Executive Director, Chester County Water Resources Authority, the sluice gate is operated at least twice a year by the Pennsylvania Fish Commission, which has jurisdiction over the reservoir.

**1.3 Pertinent Data**

**a. Drainage Area.**

Square Miles	2.94
--------------	------

**b. Discharge at Dam Site. (cfs)**

Max. flood of record (estimated)	60
Max. principal spillway discharge (water level to emergency spillway crest Elev. 619.5)	101.5
Max. emergency spillway discharge (water level to top of dam - Elev. 625.5)	6,050
Max. non-overtopping discharge	6,165

**c. Elevation (feet above MSL)**

Principal spillway crest (normal pool)	615.8
Emergency spillway crest	619.5
Top of dam (design)	625.5
Upstream berm	612.8
Pond drain invert	595.25
Principal spillway conduit invert (inlet)	594.0
Principal spillway conduit invert (outlet)	592.0
Streambed at downstream toe	594.5

**d. Reservoir (miles)**

Length of normal pool	0.57
Length of flood storage pool	0.68
Length of maximum (non-overtopping) pool	0.76

**e. Storage (acre-feet)**

Sediment, fish, and wildlife (normal pool)	1,025
Floodwater retardation	596
Total (emergency spillway crest)	1,621
Top of dam	2,880

**f. Reservoir Surface Area (acres)**

Normal pool	146
Flood storage pool	183
Top of dam	245

**g. Dam Data**

Type	Compacted Earth
Length	1,500 feet
Height	31 feet (Maximum)
Crest Width	14 feet
Side slopes	3H:1V (upstream) 2.5H:1V (downstream)

Zoning	2 zones separated by a chimney drain (refer to section 1.2.a)
Impervious core	Impervious upstream and central section (refer to section 1.2.a)
Cutoff Grout Curtain	Yes (refer to section 1.2.a) None

#### h. Spillways

##### Principal

Type	Drop inlet structure with 30-inch conduit and impact basin
Crest Width	15 feet
Crest Elevation	615.8
Gates	None
Upstream Channel	None
Downstream Channel	Impact basin directs flow into a 300 foot long trapezoidal channel that is riprap lined for the initial 20 feet.

##### Emergency

Type	Trapezoidal vegetated earth cut (3H:1V side slopes)
Crest Width	150 feet
Crest Elevation	619.5
Gates	None
Upstream Channel	240-foot long approach section on a 2% grade
Downstream Channel	300-foot long discharge channel on a 3.3% grade

#### i. Outlet Works

Type	18-inch reinforced concrete pipe pond drain connected to the upstream face of the riser.
Length	50 feet
Closure	Sluice gate mounted on the inside face of the riser.
Access	Control mechanism is located on top of the riser and is accessible from the upstream face of the dam.

Regulating Facilities

Hand operated gate

SECTION 2  
ENGINEERING DATA

2.1 Design

- a. Data Available. The information available for review of Struble Dam includes the following obtained from DER:
  - 1. "Application", "Report Upon the Application", and "Permit" to construct Robert G. Struble Dam, DER, 1970.
  - 2. Complete set of "As-Built" plans including soil boring logs
  - 3. Set of original design drawings
  - 4. SCS design report
  - 5. Construction progress reports and photographs
  - 6. Annual inspection reports (1972 through 1974)
  - 7. Miscellaneous correspondence and memoranda
- b. Design Features. The design features are described in Section 1.2.a and shown on the Plates in Appendix E.

2.2 Construction

Based on the field investigation and the information available in the construction reports, the dam appears to have been constructed in general conformance with the design drawings.

2.3 Operation

Operational procedures are limited to the control of the sluice gate for the pond drain. The sluice gate is located on the inside face of the upstream endwall of the riser.

2.4 Evaluation

- a. Availability. The information utilized in this report was provided by the DER.
- b. Adequacy. The information concerning design and construction supplied by the DER is adequate for a Phase I investigation.
- c. Validity. There is no reason to question the validity of the data obtained from DER.

SECTION 3  
VISUAL INSPECTION

3.1 Findings

- a. General. The field inspection of Struble Dam took place on May 2, 1979. At the time of inspection, the water surface was approximately two inches above the principal spillway crest. The observations and comments of the field inspection team are in the check list which is Appendix B of this report. The appearance of the facility indicated that the dam and its appurtenances are well maintained.
- b. Dam. On the date of the inspection, the upstream and downstream faces of the earth embankment appeared to be in good condition and supported a thick grass cover. The upstream berm and riprap lining are composed of large, dumped rock (up to 4-foot in size) with little apparent gradation.

A large, wet area, approximately 200 feet square was observed beginning about 100 feet downstream of the embankment between the emergency spillway and the principal spillway outlet channel. Several smaller marshy areas were noted between the right abutment and the principal spillway outlet channel. These wet areas appear to be natural marshy ground and not the result of seepage through or beneath the embankment. According to Mr. David Yaeck, Executive Director of the Chester County Water Resources Authority, the marshy ground existed prior to the construction of Struble Dam.

- c. Appurtenant Structures. Mr. Yaeck stated that in 1972 an under drain was installed downstream of the embankment between the right abutment and the principal spillway outlet channel. The purpose of the drain is to collect ground water and direct the flow into the spillway outlet channel. The under drain outlet was clogged with debris; however, during the inspection, when debris was removed, clear water discharged freely from the drain.

The principal spillway appears to be in good condition and is apparently functioning properly. During the inspection, a small amount of water was flowing from the toe drain outlets, which are located in the impact basin side walls. The discharge was slightly greater from the left side toe drain.

The emergency spillway has a thick grass cover throughout its length. A vegetated earth dike extends over 100 feet downstream along the right side of the emergency spillway providing adequate protection of the embankment from emergency spillway discharge.

- d. Reservoir Area. A railroad embankment, approximately 500 feet long is constructed across the northwest tip of the reservoir, about 3,000 feet from the dam. A large culvert is built through the railroad embankment. The hydrology of the reservoir does not appear to be affected by the culvert. The slopes along the perimeter of the reservoir are relatively mild and well vegetated.
- e. Downstream Channel. A trapezoidal cut channel with a 10-foot bottom width and 2H:1V side slopes joins the natural streambed about 300 feet downstream of the dam. The stream flows through the town of Cupola 1.5 miles downstream of the dam. There are approximately 5 homes and 25 people in the potential damage area in Cupola. Failure of Struble Dam would cause extensive property damage and probable loss of life.

## SECTION 4

### OPERATIONAL PROCEDURES

#### **4.1 Procedures**

According to Mr. Yaeck, the pond drain sluice gate is operated at least twice a year by the Pennsylvania Fish Commission, which has jurisdiction over the reservoir.

#### **4.2 Maintenance of the Dam**

Maintenance inspections are performed four times a year by the Chester County Water Resources Authority. General maintenance, such as cutting the grass, is performed once a year.

#### **4.3 Maintenance of Operating Facilities**

The Pennsylvania Fish Commission maintains possession of the sluice gate hoist wheel. The hoist is lubricated as required and the gate is operated periodically.

#### **4.4 Warning Systems in Effect**

Stream elevations of Brandywine Creek are monitored by gages and the residents of the area are kept informed by emergency broadcasts on local commercial radio stations during periods of heavy rainfall. According to Mr. Yaeck, residents downstream of Struble Dam would be notified by this radio network as soon as The Emergency Spillway went into operation.

#### **4.5 Evaluation of Operational Adequacy**

Struble Dam has a well established maintenance program and appears to be carefully monitored. The operational features and procedures are considered adequate.

The Dam is not readily accessible under severe weather conditions for inspection and emergency action.

SECTION 5  
HYDRAULICS AND HYDROLOGY

5.1 Evaluation of Features

a. Design Data. The complete SCS hydraulic and hydrologic design is available from DER. A summary of the SCS hydrologic and hydraulic design is enclosed in Appendix C, Sheet 10.

Struble Dam has a drainage area of 2.9 square miles and impounds a reservoir with a normal pool storage capacity of 1,025 acre-feet. Additional storage capacity to the crest of the emergency spillway of 596 acre-feet is available for flood control. The spillway system consists of a principal spillway (drop inlet closed conduit spillway) and an emergency spillway (vegetated earth cut). The combined spillway discharge capacity without overtopping of the embankment is 6,165 cfs.

b. Experience Data. No rainfall or reservoir level records are kept for this dam. Mr. Yaeck stated that the highest reservoir level occurred in January of 1978 when the reservoir level was about 1.5 feet above the principal spillway crest.

c. Visual Observations. On the date of the inspection, the principal spillway and the emergency spillway appeared to be unobstructed and functioning as designed. Further observations are given in Appendix B.

d. Overtopping Potential. The Spillway Design Flood (SDF) for this "Intermediate" size "High" hazard dam is the full PMF. The PMF hydrograph was routed through the reservoir with the starting water surface elevation at the crest of the principal spillway, Elevation 615.8. Based on the hydraulic and hydrologic analysis, the spillway system is capable of discharging 100% of the PMF without overtopping of the embankment. The peak inflow and outflow rates for the PMF are 8,453 cfs and 5,592 cfs respectively (see Appendix C for computations).

e. Spillway Adequacy. The Struble Dam spillway system is classified as "Adequate".

SECTION 6  
STRUCTURAL STABILITY

6.1 Evaluation of Structural Stability

a. Visual Observations. The downstream face of the embankment and the visible portion of the upstream face appear to be in good condition. There are no visible signs of instability of the dam. The survey of the top of dam revealed that little settlement has occurred since construction. The entire length of the top of dam is well above design elevation which is evidence of the overfill built into the top of the dam for possible future settlement of the embankment and to maintain a level appearance.

The structural features of the principal spillway appear to be in good condition. There is no evidence of cracking or spalling of the concrete surfaces. The emergency spillway shows no visible signs of structural instability.

b. Design and Construction Data. The SCS design report, construction progress reports and photographs were obtained from DER. "As-Built" plans were also included in the DER files.

The earth embankment cross section geometry shown on the drawings was verified during the field inspection. The embankment and foundation design is consistent with SCS, Soil Mechanics Laboratory Report and Recommendations.

SCS inspection personnel were reported to be present on the job site during the entire construction operation.

c. Operating Records. There are no operating records maintained for this structure.

d. Post-Construction Changes. The only post-construction change was the installation of the under drain downstream of the right side of the embankment. The drain was added in 1972, one year after construction was completed, to drain ground water from this area.

e. Seismic Stability. Struble Dam is located in Seismic Zone I on the Seismic Zone Map of Contiguous States. A dam located in Zone I is generally considered to be safe under any expected earthquake loading, if it is stable under static loading conditions.

## SECTION 7

### ASSESSMENT, RECOMMENDATIONS, AND PROPOSED REMEDIAL MEASURES

#### 7.1 Dam Assessment

- a. Safety. The visual observations and review of available information indicate that the Robert G. Struble Dam is in good condition. The embankment and appurtenant structures show no signs of instability or structural deficiency. The dam is inspected regularly and is well maintained.

The combined spillway system is capable of discharging 100% of the PMF without overtopping of the embankment. Therefore, the spillway system is classified as hydraulically "Adequate".

- b. Adequacy of Information. The information received from DER is adequate for a Phase I investigation.
- c. Urgency. Regular inspections of the structure should be continued.
- d. Necessity for Further Investigation. Further investigations are not considered necessary.

#### 7.2 Recommendations and Proposed Remedial Measures

##### a. Facilities

1. The dam and appurtenant structures are considered to be in good condition and no remedial measures are recommended at the present time.

##### b. Operation and Maintenance Procedures

1. Struble Dam is regularly inspected and general maintenance is performed on an annual basis. The sluice gate is operated periodically and maintenance to the gate system is performed as required. Emergency broadcasts on local commercial radio stations inform area residents of high stages in the Brandywine Creek. Residents downstream of Struble Dam would be alerted when the emergency spillway goes into operation.

APPENDIX

A

Check List Engineering Data  
Design, Construction, Operation  
Phase I

Sheet 1 of 4

**REMARKS**

ITEM AS-BUILT DRAWINGS

## A COMPLETE SET OF AS BUILT PLANS WERE OBTAINED

## REGIONAL VICINITY MAP

REFER TO APPENDIX E, PLATE 1

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CONSTRUCTION PROGRESS REPORTS AND PHOTOGRAPHS  
ARE AVAILABLE FROM DFR.

## TYPICAL SECTIONS OF DAM

OUTLETS - PLAIN  
DETAIL

RAINFALL/RESERVOIR RECORDS

INCLUDES AS COUNT PLANS

CONTAINS NO HAZARDOUS SUBSTANCES.

NONE AVAILABLE

Sheet 2 of 4

ITEM	REMARKS
DESIGN REPORTS	SCS DESIGN REPORT WAS OBTAINED FROM DER.
GEOLOGY REPORTS	CONTAINED IN SCS DESIGN REPORT
DESIGN COMPUTATIONS HYDROLOGY & HYDRAULICS DAM STABILITY SEEPAGE STUDIES	CONTAINED IN SCS DESIGN REPORT
MATERIALS INVESTIGATIONS BORING RECORDS LABORATORY } FIELD }	BORING LOGS ARE CONTAINED IN THE AS BUILT PLANS, SOILS REPORT IS PART OF THE DESIGN REPORT
POST-CONSTRUCTION SURVEYS OF DAM	CONTAINED IN AS BUILT PLANS
BORROW SOURCES	SHOWN IN AS BUILT PLANS

ITEM	REMARKS
MONITORING SYSTEMS	NONE
MODIFICATIONS	NO INFORMATION WAS AVAILABLE FROM PER CONCERNING THE FRENCH DRAIN. THE REPORTED INFORMATION WAS COMMUNICATED BY MR. GARY YACK.
HIGH POOL RECORDS	NONE AVAILABLE
POST CONSTRUCTION ENGINEERING STUDIES AND REPORTS	NONE
PRIOR ACCIDENTS OR FAILURE OF DAM DESCRIPTION REPORTS	NONE
Maintenance OPERATION RECORDS	NONE AVAILABLE

ITEM	REMARKS
SPILLWAY PLAN	
SECTIONS	REFER TO APPENDIX E
DETAILS	
OPERATING EQUIPMENT PLANS & DETAILS	REFER TO APPENDIX E
MISCELLANEOUS	REFER TO SECTION 2.1.A

APPENDIX

B

Check List

Visual Inspection

Phase I

CHECK LIST  
VISUAL INSPECTION  
PHASE I

Sheet 1 of 11

Name Dam	<u>Robert G. Sprague Dam</u>	County	<u>CHESTER</u>	State	<u>PENNA.</u>	National ID #	<u>PA 00621</u>
Type of Dam	<u>EARTH</u>	Hazard Category _____					
Date(s) Inspection	<u>5/2/79</u>	Weather	<u>FAIR</u>	Temperature	<u>65° F.</u>		

Pool Elevation at Time of Inspection 616 ± M.S.L. Tailwater at Time of Inspection 593 ± M.S.L.

Inspection Personnel:

<u>Leroy H. DeHerr</u>	<u>Leroy H. DeHerr</u>	<u>Robert R. Powers</u>
_____	_____	_____

Lee H. DeHerr Recorder

Remarks:

Mr. Gary Yaeck and Mr. Al Palmatter, REPRESENTATIVES FROM THE CHESTER COUNTY WATER RESOURCES AUTHORITY, WERE PRESENT DURING THE INSPECTION. MR. GARY EMANUEL, FROM THE DEPARTMENT OF ENVIRONMENTAL RESOURCES, WAS ALSO PRESENT.

CONCRETE/MASONRY DAMS

SHEET 2 OF 11  
VISUAL EXAMINATION OF                   OBSERVATIONS                   REMARKS OR RECOMMENDATIONS

ANY NOTICEABLE SEEPAGE

N/A

STRUCTURE TO  
ABUTMENT/EMBANKMENT  
JUNCTIONS

N/A

DRAINS

N/A

WATER PASSAGES

N/A

FOUNDATION

N/A

CONCRETE/MASSONRY DAMS

VISUAL EXAMINATION OF		OBSERVATIONS	REMARKS OR RECOMMENDATIONS
SURFACE CRACKS	CONCRETE SURFACES	N/A	
STRUCTURAL CRACKING		N/A	
VERTICAL AND HORIZONTAL ALIGNMENT		N/A	
MOROLITH JOINTS		N/A	
CONSTRUCTION JOINTS		N/A	

Sheet 4 of 11

	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
--	--------------	----------------------------

EMBANKMENT

VISUAL EXAMINATION OF

SURFACE CRACKS

None observed

UNUSUAL MOVEMENT OR  
CRACKING AT OR BEYOND  
THE TOE

None observed

SLoughing or Erosion of  
embankment and abutment  
slopes

None observed

VERTICAL AND HORIZONTAL  
ALIGNMENT OF THE CREST  
revealed that little settlement  
had occurred and the top of dam  
showed a definite camber (as  
constructed).

RIPRAP FAILURES

None observed

EMBANKMENT

Sheet 5 of 11

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
DRAINS	The Two Main Outlets And The Frontal Gage Outlet Were DRAINING CLEAR WATER. The Drains Were Located beneath The Pavement Of The Main Gage Street.	
JUNCTION OF EMBANKMENT AND ABUTMENT, SPILLWAY AND DAM	No Evidence of Seepage.	None
ANY NOTICEABLE SEEPAGE		
STAFF GAGE AND RECORDER		None

OUTLET WORKS

PRINCIPAL STREAM

Sheet 6 of 11

<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
CRACKING AND SPALLING OF CONCRETE SURFACES IN OUTLET CONDUIT	THE OUTLET CONDUIT IS LOCATED PERPENDICULAR TO THE FLOORING. THE OUTLET CONDUIT IS LOCATED PERPENDICULAR TO THE FLOORING.	THE INTAKE CONDUIT (RISER) APPEARED TO BE IN GOOD CONDITION AND WAS UNSTRUCTURED.
INTAKE STRUCTURE		THE IMPACT RISER ALSO APPEARED IN GOOD CONDITION AND WAS FUNCTIONING PROPERLY.
OUTLET STRUCTURE		
OUTLET CHANNEL		THE INITIAL PORTION OF THE OUTLET CHANNEL IS PEELED AWAY.
EMERGENCY GATE		THE ONLY GATE IS THE SOURCE GATE WHICH CONTROLS THE POND DRAIN INLET.

UNGATED SPILLWAY  
F.M.F.O. M.C.Y. 5 H.L.L.-J.A. /

VISUAL EXAMINATION OF		OBSERVATIONS		REMARKS OR RECOMMENDATIONS	
CONCRETE WEIR		N/A			
APPROACH CHANNEL		THE FOREBAY CHANNEL IS WIDE VEGETATION AND SLOPES UNMADE ON A 2% SLOPE.			
DISCHARGE CHANNEL		THE DISCHARGE CHANNEL IS WIDE UNMADE ON A SLOPES UNMADE ON A 3.2% SLOPE.			
BRIDGE AND PIERS		N/A			

Sheet 7 of 11

GATED SPILLWAY

Sheet 8 of 11

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
CONCRETE SILL	N/A	

APPROACH CHANNEL

DISCHARGE CHANNEL

BRIDGE AND PIERS

GATES AND OPERATION  
EQUIPMENT

N/A

N/A

N/A

INSTRUMENTATION

VISUAL EXAMINATION	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
MONUMENTATION/SURVEYS		

MONUMENTATION/SURVEYS

N/A

OBSERVATION WELLS

N/A

WEIRS

N/A

PIEZOMETERS

N/A

OTHER

N/A

RESERVOIR

Sheet 10 of 11  
VISUAL EXAMINATION OF  
OBSERVATIONS      REMARKS OR RECOMMENDATIONS

SLOPES

THE EFFECTIVE SLOPES ARE  
MILD AND GRADUALLY CHANGED.

SEDIMENTATION

THE DESIGN ALLOWED FOR  
50 YEARS OF SEDIMENTATION  
STORAGE IN THE NORMAL FLOW  
STRUCTURE.

DOWNSTREAM CHANNEL

VISUAL EXAMINATION OF  
Sheet 11 of 11

CONDITION (OBSTRUCTIONS, DEBRIS, ETC.)	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
	No obstructions or debris was observed in the down- stream channel rock riffle hundred feet downstream	
SLOPES	The downstream channel slopes about 1% per cent 7.16 day and 7.17 Curva	
APPROXIMATE NO. OF HOMES AND POPULATION	Tropic has about 4 homes and 20 people located in the downstream town of Curva.	

APPENDIX

C

Hydrologic & Hydraulic Data

TABLE OF CONTENTS - APPENDIX C

HYDROLOGIC & HYDRAULIC DATA

PMP CALCULATIONS	SHEET 1
SNYDER COEFFICIENTS	SHEET 1
STAGE-STORAGE	SHEET 2
STAGE-DISCHARGE	SHEETS 2-3
HEC-1 DAM SAFETY VERSION COMPUTER OUTPUT	SHEETS 4-9
SCS DESIGN SUMMARY	SHEET 10



O'BRIEN & GERE

SUBJECT	SHEET	BY	DATE	JOB NO
ROBERT G. STRUBLE DAM	1	RRB	3/30/79	

HYDROLOGY CALC'S.

DRAINAGE AREA : 2.94 mi.<sup>2</sup>

PMP CALCULATIONS (HM REPORT 33)

AREA IS IN ZONE 6

24 HR., 200 SQ. MI. RAINFALL = 23.5"

<u>HR.</u>	<u>%</u>	<u>RAINFALL</u>	<u>LRF</u>
6	113	26.6"	26.6"
12	123	28.9"	2.3"
24	132	31.0"	2.1"
48	142	33.4"	2.4"

SNYDER COEFFICIENTS

FROM INFO. PROVIDED BY COE, FOR THE  
DELAWARE RIVER BASIN, ZONE 10 :

$$C_p = 0.60$$

AND

$$C_t = 1.25$$

$$t_p = C_t (L \cdot Lca)^{0.3}$$

$$L = 2.6 \text{ miles}$$

$$Lca = 1.0 \text{ miles}$$

$$t_p = 1.25 (2.6 \cdot 1.0)^{0.3} =$$

$$1.66 \text{ HRS.}$$



OBRIEN &amp; GERE

SUBJECT		SHEET	BY	DATE	JOB NO
STRUBLE DAM		2			

STAGE-STORAGE CURVE VALUES (FROM SCS DESIGN REPORT)

<u>ELEVATION</u>	<u>STORAGE (ACRE-FT.)</u>
601	0 *
615.8	991
619.5	1587
625.5	2846

\* STORAGE VALUES ARE WITHOUT THE 33.8 ACRE-FEET OF SEDIMENT STORAGE.

STAGE-DISCHARGE CURVE VALUES (FROM SCS DESIGN REPORT)

$$\text{WEIR FLOW} \rightarrow Q_w = CLH_w^{3/2} = 3.1(15) H_w^{3/2} = 46.5 H_w^{3/2}$$

$$Q_d = 22.95 H_p^{1/2}$$

<u>STAGE</u>	<u>H<sub>w</sub></u>	<u>Q<sub>w</sub></u>	<u>H<sub>p</sub></u>	<u>Q<sub>p</sub></u>	<u>H<sub>es</sub></u>	<u>Q<sub>es</sub></u>	<u>Q<sub>total</sub></u>
615.8	0	0					0
616.2	0.4	11.8					11.8
616.6	0.8	33.3					33.3
617.0	1.2	61.1					61.1
617.43	1.63	95.5	17.43	96			95.5
618.0			18	97.3			97.3
618.5			18.5	98.6			98.6
619.0			19	100			100
619.5			19.5	101.5	0	0	101.5
621.26			21.26	105.8	1.76	754.55	860
622.15			22.15	108.1	2.65	1514.6	1623
622.89			22.89	109.7	3.39	2278.5	2388
			(CONT)				



O'BRIEN & GERE

SUBJECT	SHEET	BY	DATE	JOB NO
	3			

STAGE - DISCHARGE (CONT.)

<u>STAGE</u>	<u>H.P.</u>	<u>Q.P.</u>	<u>tES</u>	<u>tFS</u>	<u>Q TOTAL</u>
623.54	23.54	111.3	4.04	3047.0	3158
624.64	24.64	113.8	5.14	4591.6	4705
625.64	25.64	116.1	6.14	6148.0	6264
626.5	26.5	118.2	7.0	7715.0	7833

FEDERAL HAZARD MITIGATION PROGRAM

DAM SAFETY INSPECTION REPORT

JULY 1976

LAST MODIFICATION: 25 SEP 78

## NATIONAL DAM INSPECTION PROGRAM

				ROUTING G. STABILIZABLE DAM				
				PERF IN SIGHTED	PERF IN OBSERVED			
1	2	4.2	4.3	0	30	0	0	0
2	3	150	0	1	1	0	0	0
3	4	3	1	9	1	0	0	0
4	5	1	1	9	1	0	0	0
5	6	1	1	9	1	0	0	0
6	7	1	1	9	1	0	0	0
7	8	1	1	9	1	0	0	0
8	9	0	0	1	1	0	0	0
9	10	0	0	1	1	0	0	0
10	11	0	0	1	1	0	0	0
11	12	0	0	1	1	0	0	0
12	13	0	0	1	1	0	0	0
13	14	1.66	0.60	2	1.0	0.05	0.05	0.05
14	15	1.65	0.65	2	1.0	0.05	0.05	0.05
15	16	1.65	0.65	2	1.0	0.05	0.05	0.05
16	17	1.65	0.65	2	1.0	0.05	0.05	0.05
17	18	1.65	0.65	2	1.0	0.05	0.05	0.05
18	19	1.65	0.65	2	1.0	0.05	0.05	0.05
19	20	1.622+15	622+89	616+2	616+6	617+7	617+3	615+8
20	21	1.622	622	623+54	623+54	624+54	625+54	618+5
21	22	1.623	623	1.2	1.3	1.1	1.1	1.1
22	23	1.623	623	2.88	2.158	4.705	6.264	6.265
23	24	1.623	623	9.91	1.587	2.845	7.633	8.60
24	25	1.623	623	615+8	619+5	625+5	625+5	619+5
25	26	1.623	623	625+5	2.1	1.5	1.5	1.5
26	27	1.623	623	625+5	2.1	1.5	1.5	1.5



## SHEET 6

Q(1)W 24.82 22.97 1.05 91533,  
 ( 4.30. ) ( 5.93. ) ( 47.1 ) 2591.93 )

## HYDROGRAPH ROUTING

## ROUTING THROUGH STUPIDE LAKE

	IStage	ICOMP	IECON	ITAPE	JPLT	IPRT	INAME	IStage	IStage	IAUTO
	OUTFLD	1	0	0	0	0	0	0	0	0
GLOSS	CLOSS	Avg	ROUTING DATA							
0.0	0.000	0.00	IRFS ISAWC							
NSTPS	NSTOL	LAG	AMSKX	X	TSK	STORA	ISPRAT			
1	0	0	0.000	0.000	0.000	-616.	-1			
STAGE	615.80	616.20	616.60	617.00	617.43	618.00	619.00	619.50	621.26	
	622.15	622.89	623.54	624.64	625.64	626.50				
FLO+	0.00	12.00	33.00	61.00	96.00	97.00	99.00	100.00	102.00	104.00
CAPACITY=	1623.00	2368.00	3158.00	4705.00	6264.00	7833.00				
ELEVATION=	0.	991.	1597.	2846.						
CREL	SPRID	CODW	EXPW	ELEV	COOL	CAREA	EMPL			
615.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
PEAK OUTFLOW IS	158.	AT TIME	23.00	HOURS						
PEAK OUTFLOW IS	602.	AT TIME	21.00	HOURS						
PEAK OUTFLOW IS	1140.	AT TIME	20.50	HOURS						
PEAK OUTFLOW IS	1025.	AT TIME	20.00	HOURS						
PEAK OUTFLOW IS	2528.	AT TIME	19.50	HOURS						
PEAK OUTFLOW IS	3298.	AT TIME	19.50	HOURS						
PEAK OUTFLOW IS	4058.	AT TIME	19.50	HOURS						
PEAK OUTFLOW IS	4789.	AT TIME	19.00	HOURS						
					TOPEL	DAM DATA				
					625.5	600.0	EXPD	DAMWD		
						3.1	1.5	1500.		

SHEET 7

PEACE CUTELDO • 15 SASSA • AT TIME 19-09-2013

## SHEET 8

PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS  
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)  
 AREA IN SQUARE MILES (SQUARE KILOMETERS)

OPERATION	STATION	AREA	PLAN	RATIO 1 .20	RATIOS APPLIED TO FLOWS					RATIO 8 .90	RATIO 9 1.00
					RATIO 2 .30	RATIO 3 .40	RATIO 4 .50	RATIO 5 .60	RATIO 6 .70		
HYDROGRAPH AT INFLOW	2.94 ( 7.61)	1 ( 47.87)	1691. 2536.	3381. 95.74)(	4226. 119.67)(	5072. 143.61)(	5917. 167.54)(	6762. 191.48)(	7607. 215.41)(	8453. 239.35)	
ROUTED TO OUTFLOW	2.94 ( 7.61)	1 ( 4.48)	158. 602. 17.05)(	1140. 32.29)(	1825. 51.69)(	2528. 71.57)(	3288. 93.11)(	4058. 114.91)(	4789. 135.60)(	5592. 158.34)	

9 SHEET V

SISAKTANASUTA 30

PLAN	ELEVATION	STORAGE	OUTFLOW	INITIAL VALUE	SPIRIT CHEM.	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE ACRES	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	611.80	991.	0.	615.0	625.50	619.63	0.00	1614.	158.	0.00	23.00	0.00
						620.66	0.00	1831.	602.	0.00	21.00	0.00
						621.19	0.00	2025.	1140.	0.00	20.50	0.00
						622.35	0.00	2184.	1825.	0.00	20.00	0.00
						623.01	0.00	2323.	2528.	0.00	19.50	0.00
						623.63	0.00	2454.	3288.	0.00	19.50	0.00
						624.18	0.00	2569.	4058.	0.00	19.50	0.00
						624.69	0.00	2677.	4789.	0.00	19.00	0.00
						625.01	0.00	2777.	5592.	0.00	19.00	0.00
						625.49	0.00	2785.	5592.	0.00	19.00	0.00
						625.81	0.00	2785.	5592.	0.00	19.00	0.00

## SHEET 10

## U. S. DEPARTMENT OF AGRICULTURE - SOIL CONSERVATION SERVICE

Element of structure	Determining Factor	Elevation	Surface Area Acres	Storage acre-feet	Volume Inches*	Inflow Rate c.f.s.	Peak Outflow c.f.s.
50 year sediment accumulation	601.0	650	29	0.85			
Crest of riser	Sediment storage	650	1025 1054	6.54			
Crest of emergency spillway	100 year frequency storm, moisture condition II	618.5	182	3.8	5.2	102	
Design height water	X value from ES-1020 Sh. A moisture condition II	621.1 1.6'	905	577	6.77	12.73	650
Top of dam	X value from ES-1020 Sh. A moisture condition II	625.5 4.4 6.0	105	105	12.48	12.880	6050

\* Volume expressed in inches of runoff from controlled watershed area of 1802 acres.  
 \*\* Refer to hydrologic criteria in National Engineering Memorandum SCS-27  
 Time required to empty flood storage is 25 days

## HYDROLOGIC &amp; HYDRAULIC DESIGN SUMMARY

APPENDIX

D

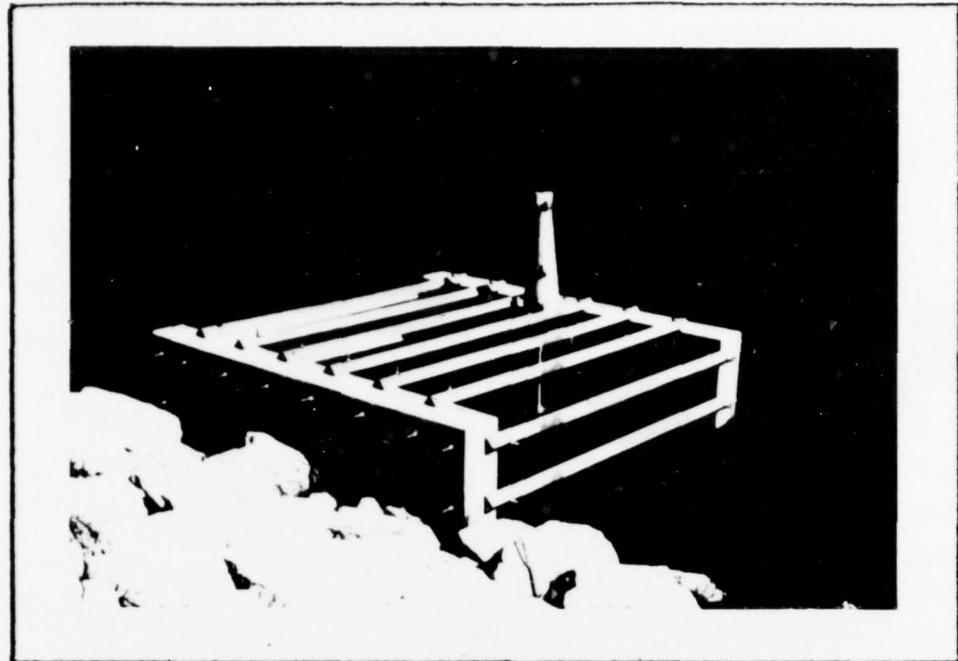
Photographs



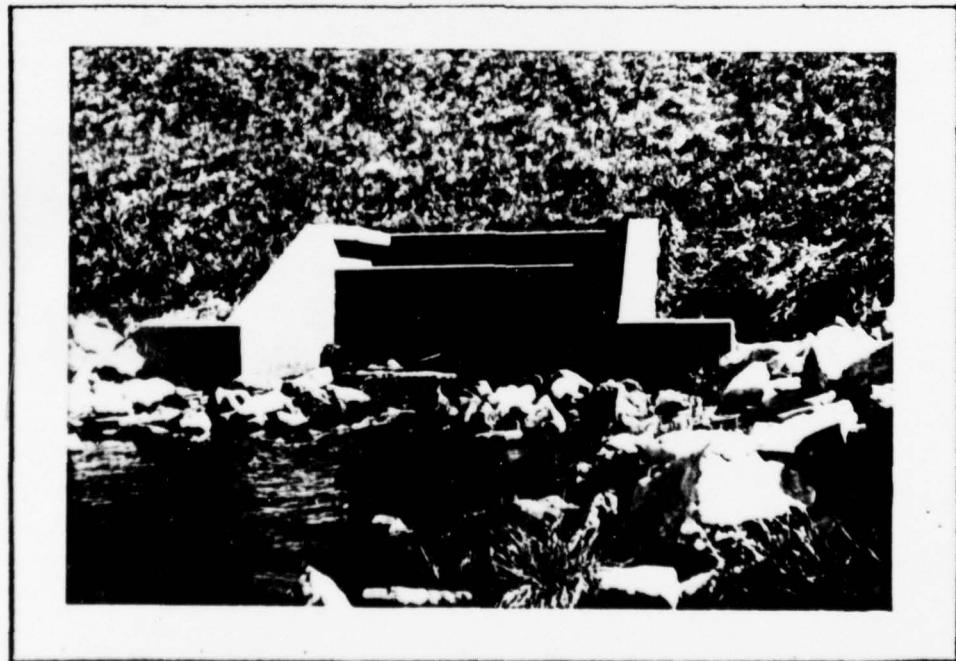
UPSTREAM FACE OF THE DAM  
LOOKING TOWARDS THE LEFT ABUTMENT



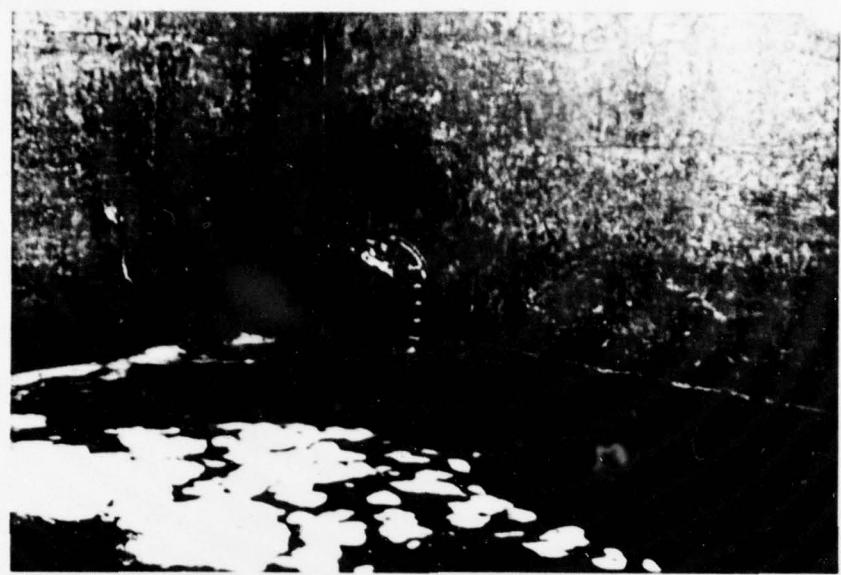
DOWNSTREAM FACE OF THE DAM  
LOOKING TOWARDS THE LEFT ABUTMENT



PRINCIPAL SPILLWAY RISER INLET STRUCTURE  
ADJACENT TO THE ROCK BERM ON THE  
UPSTREAM FACE OF THE DAM



PRINCIPAL SPILLWAY IMPACT BASIN OUTLET  
STRUCTURE AT THE DOWNSTREAM TOE OF THE DAM



OUTLET OF THE INTERNAL DRAINAGE SYSTEM  
OF THE DAM ON THE LEFT SIDE OF THE  
IMPACT BASIN



EMERGENCY SPILLWAY CHANNEL LOOKING DOWNSTREAM

$$t_p = 1.25 (2.6 + 1.0)^{0.3} = \boxed{1.66 \text{ HRS.}}$$

## APPENDIX

E

Drawings

TABLE OF CONTENTS - APPENDIX E

REGIONAL VICINITY MAP	PLATE 1
CROSS SECTION	PLATE 2
PRINCIPAL SPILLWAY	PLATE 3
GENERAL PLAN DRAWING	PLATE 4
PROFILE OF TOP OF DAM @ TIME OF INSPECTION	PLATE 5

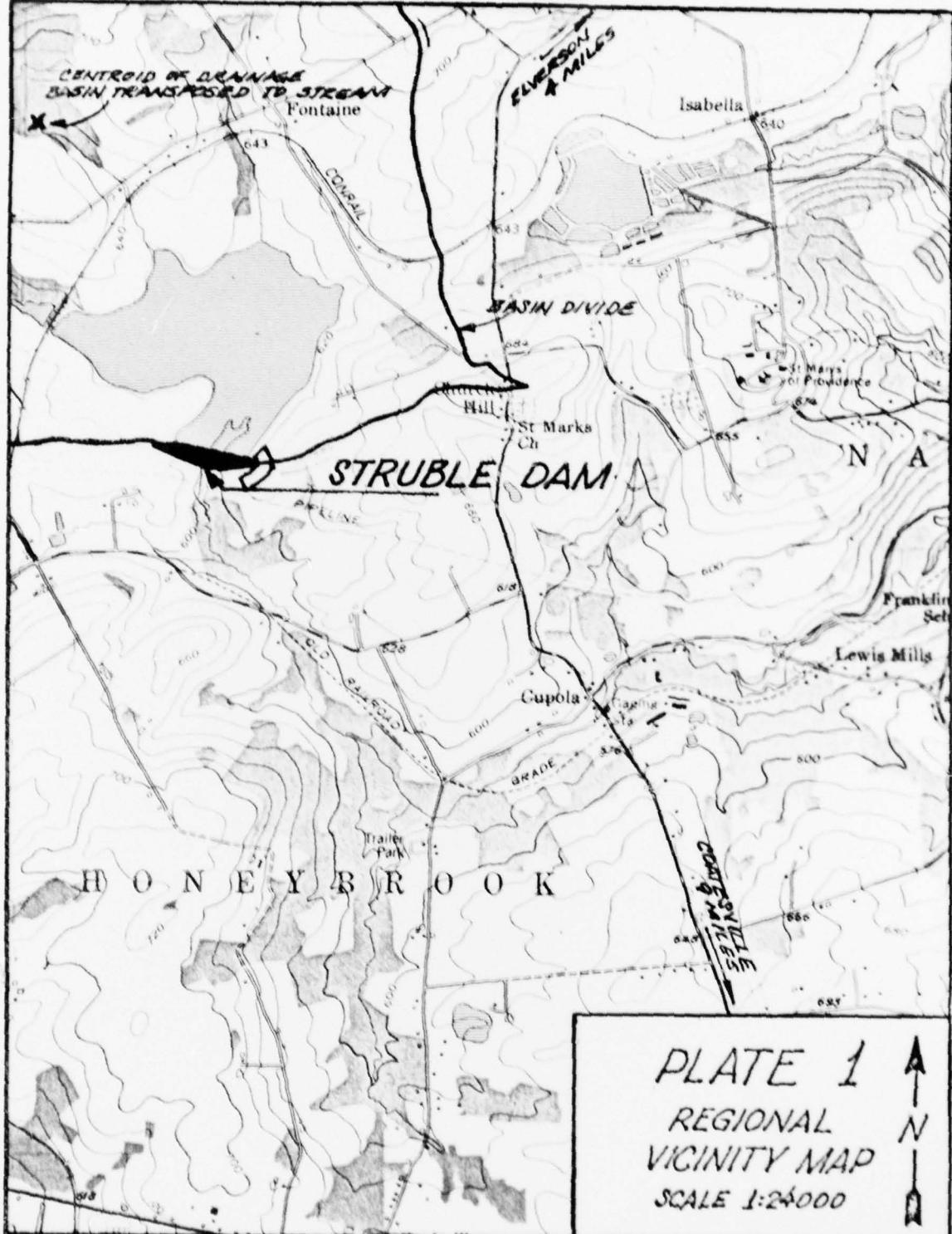
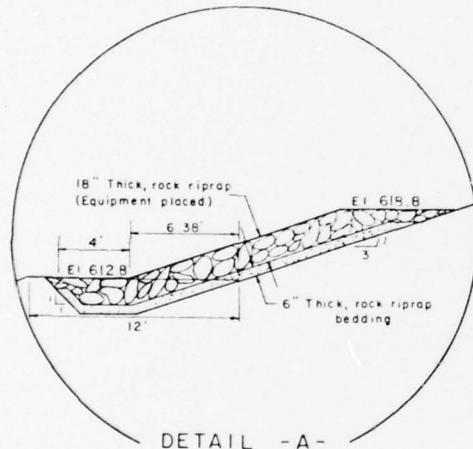
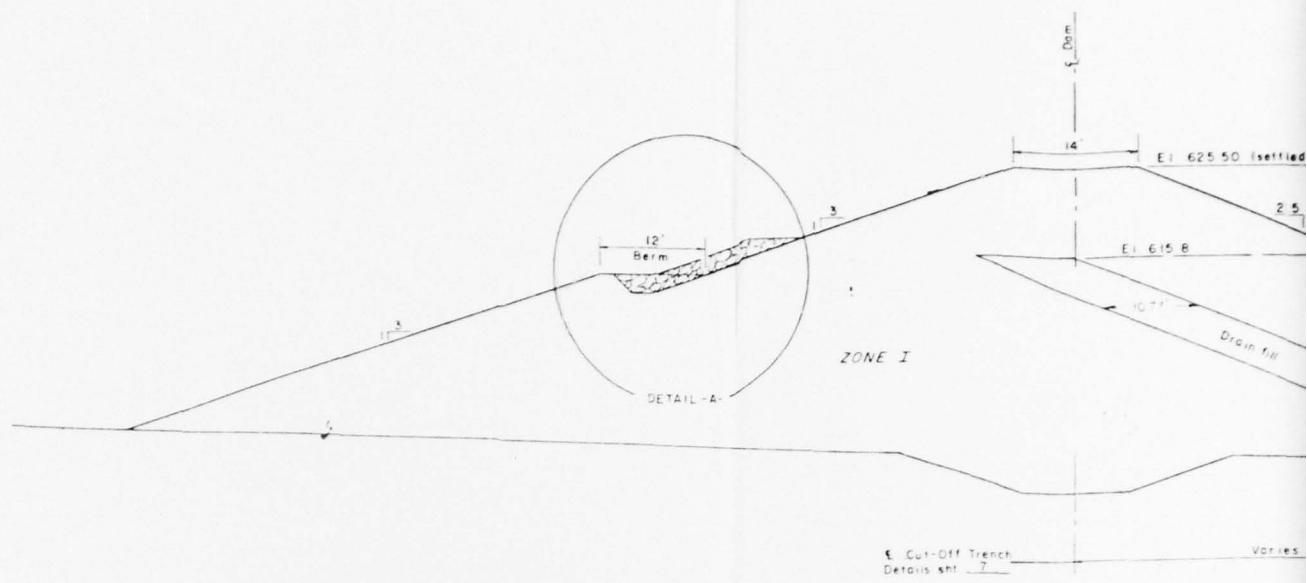


PLATE 1  
REGIONAL  
VICINITY MAP  
SCALE 1:24000



TYPICAL SECTION OF DAM

10 5 2 0 10  
SCALE in FEET

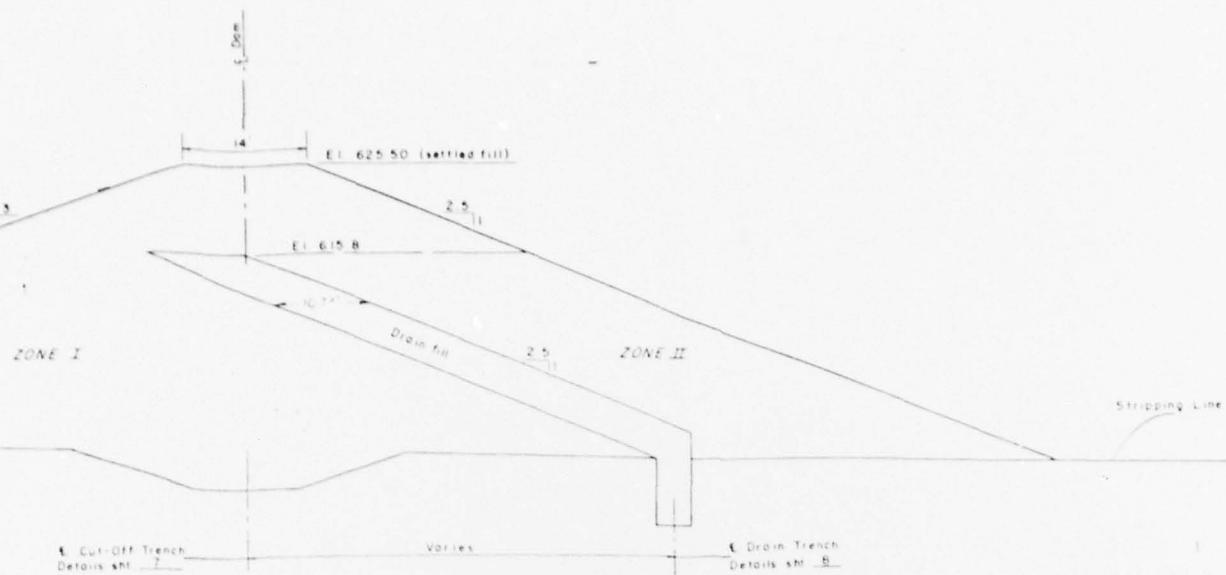
SELECTIVE PLACEMENT	MATERIAL	MAX L <sub>1</sub> ROCK SIZE	L <sub>2</sub> MAX LIFT	REQ'D L <sub>3</sub> WATER CONTENT	COMPACTI <sub>4</sub> N	
					CLASS	DEFINITION
Upstream and Cut-Off <i>ZONE I</i>	Material as represented by TP-1041, depth 1'-3", Classified as ML; by TP-122 1, depth 2'-5", Classified as ML; by TP-202 1, depth 2'-5", Classified as ML	6"	9"	Optimum + 2% - 2%	A	95 % Max density by ASTM D-698, Method "A"
Downstream <i>ZONE II</i>	Material as represented by TP-1042 depth 4'-7"; Classified as SM, by TP-202 2, depth 5'-10', Classified as SM	6"	9"	Optimum + 2% - 2%	A	95 % Max density by ASTM D-698, Method "A"

1 For fill adjacent to structures, max rock size 3"

2 Maximum permissible lift thickness before compaction

3 Water content of fill matrix at time of compaction

4 For typical compaction curves see sh<sub>t</sub> 27



TYPICAL SECTION OF DAM

10 5 2 0 10  
SCALE in FEET

CONSTRUCTION NOTES

1. Constructed slopes are
 

2.91:1	Upstream
2.43:1	Downstream
2. For constructed full elevations see sht 7

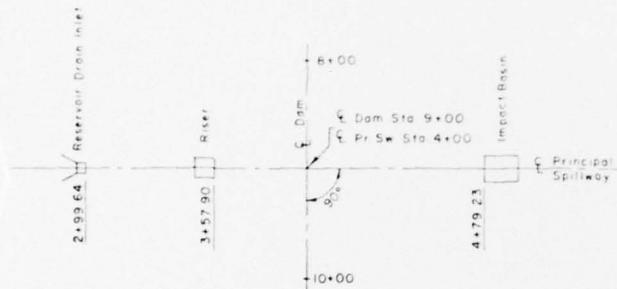
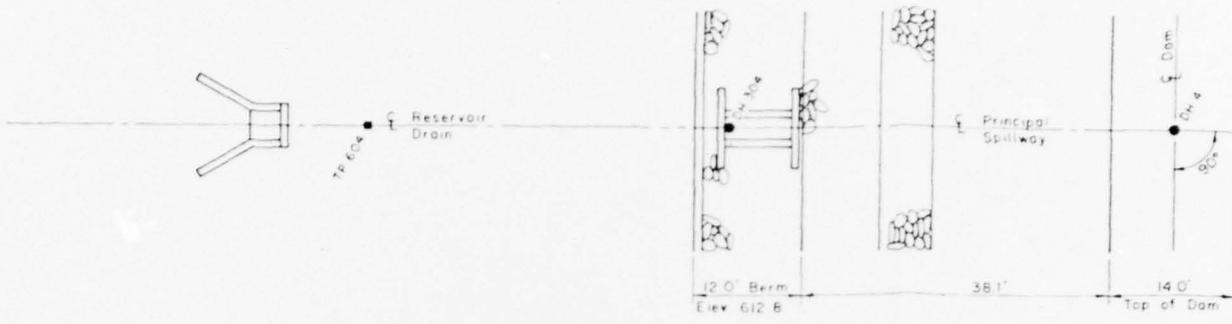
IMPACTION	14
DEFINITION	
5% Max. density by ASTM C-698, Method "A"	

5% Max. density by ASTM C-698, Method "A"

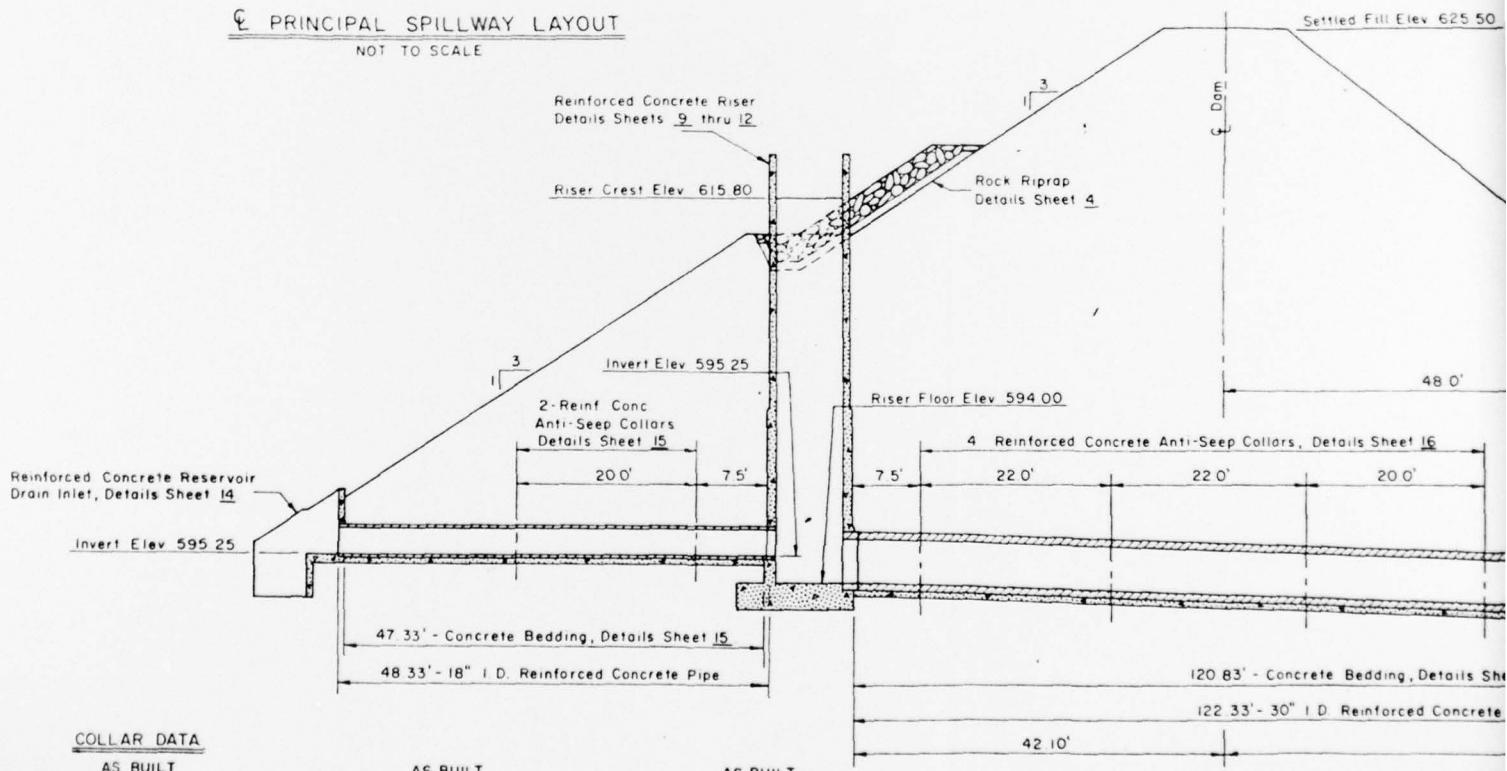
2  
**AS BUILT PLANS**

REVISIONS		
DATE	ITEM	APPROVED
4/1/69	Berm Riprap	J.A.
BRANDYWINE CREEK WATERSHED		
MULTIPLE PURPOSE DAM PA-431		
CHESTER COUNTY, PENNSYLVANIA		
FILL PLACEMENT		
U. S. DEPARTMENT OF AGRICULTURE		
SOIL CONSERVATION SERVICE		
N. Lewis Hall 4/1/69		
C. CRISSE 4/1/69		
Eng. Standard No. 4 27 PA-431-P		

PLATE 2



E PRINCIPAL SPILLWAY LAYOUT  
NOT TO SCALE



COLLAR DATA

AS BUILT  
FOR 30" ID PIPE

COLLAR	DIST. FROM RISER WALL	INVERT EL
1	7.5	593.97
2	29.5	593.87
3	51.5	593.77
4	71.5	593.67

AS BUILT  
FOR 18" ID PIPE

COLLAR	DIST. FROM RISER WALL	INVERT EL
1	7.5	593.25
2	27.5	592.25

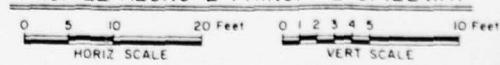
AS BUILT  
30" ID PIPE JOINT DATA

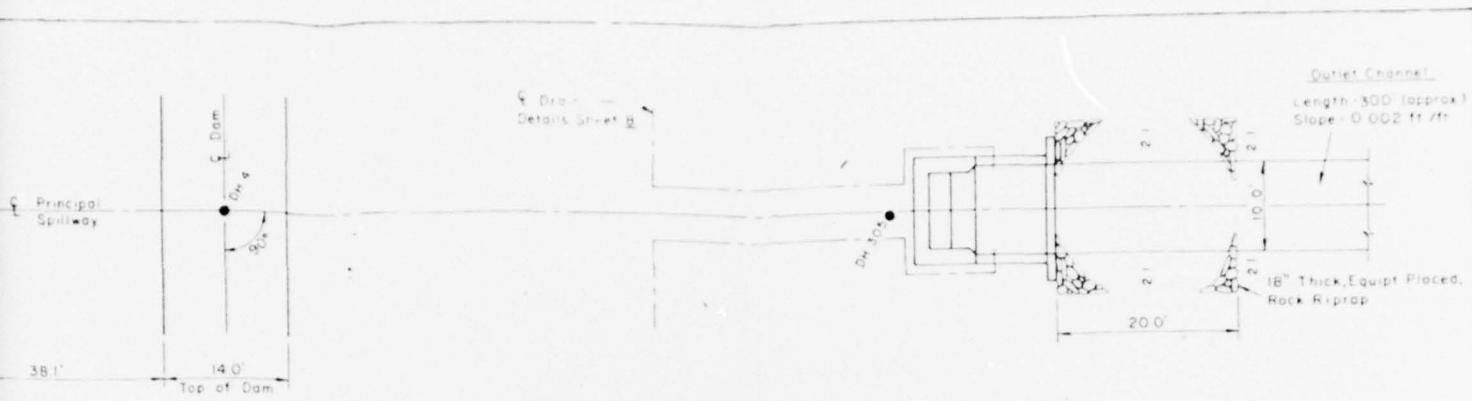
JOINT	DIST. FROM RISER WALL	INVERT EL
J-1	0.33	594.0
J-2	16.33	593.93
J-3	32.33	593.86
J-4	48.33	593.71
J-5	64.33	593.46
J-6	80.33	593.18
J-7	96.33	592.80
J-8	112.33	592.34
J-9	128.33	592.00

AS BUILT  
18" ID PIPE JOINT DATA

JOINT	DIST. FROM RISER WALL	INVERT EL
J-1	0.33	595.25
J-2	16.33	595.25
J-3	32.33	595.25
J-4	48.33	595.25

PROFILE ALONG E PRINCIPAL SPILL WAY





PLAN VIEW OF PRINCIPAL SPILLWAY

0 5 10 20 Feet  
SCALE

18" I.D. Reinforced Concrete Pressure Pipe, Steel Cylinder Type, Spec 541 (AWWA C-300 or C-301)

48' - Straight sections

1 - Bell and flange wall fitting (for 18" wall)

Pressure head = 30'

Load = 8,991 lbs per lin. ft.

Min. 3 edge bearing strength for

0.01" Crack non-prestressed pipe = 4,067 lbs per lin. ft.

0.001" Crack prestressed pipe = 3,058 lbs per lin. ft.

Based on O.D. of 2.00"

48 33' - TOTAL

30" I.D. Reinforced Concrete Pressure Pipe, Steel Cylinder Type, Spec 541 (AWWA C-300 or C-301)

122' - Straight sections

1 - Spigot ring wall fitting (for 18" wall)

Pressure head = 30'

Load = 19,534 lbs per lin. ft.

Min. 3 edge bearing strength for

0.01" Crack non-prestressed pipe = 8,837 lbs per lin. ft.

0.001" Crack prestressed pipe = 6,644 lbs per lin. ft.

Based on O.D. of 2.96"

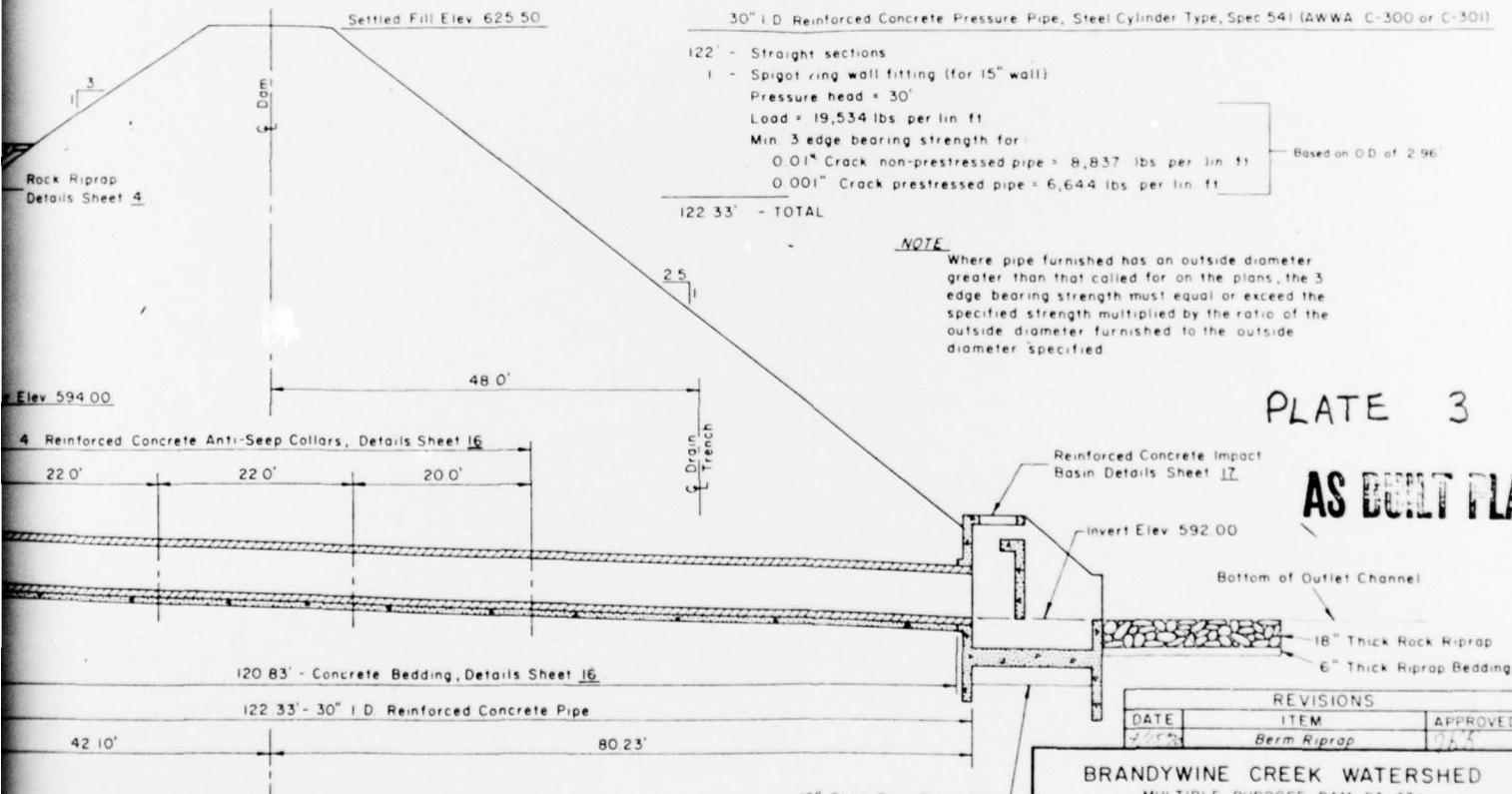
122 33' - TOTAL

NOTE

Where pipe furnished has an outside diameter greater than that called for on the plans, the 3 edge bearing strength must equal or exceed the specified strength multiplied by the ratio of the outside diameter furnished to the outside diameter specified.

PLATE 3

AS BUILT PLANS



PROFILE ALONG E PRINCIPAL SPILLWAY

0 5 10 20 Feet  
HORZ SCALE

0 1 2 3 4 5 10 Feet  
VERT SCALE

CONSTRUCTION NOTES

1. Outlet end of 30" pipe (spigot end) and inlet end of 18" pipe (bell end) to be finished so that no metal is exposed.
2. Pipe layout data will be furnished by the Engineer.
3. Riprap bedding shall meet fine drain fill gradation limits (Sheet 8).
4. For logs of test holes see sheets 19 thru 26.

REVISIONS  
DATE 11/69 ITEM APPROVED  
R. A. STALTER 11-69

BRANDYWINE CREEK WATERSHED  
MULTIPLE PURPOSE DAM PA-431  
CHESTER COUNTY, PENNSYLVANIA  
PRINCIPAL SPILLWAY

U. S. DEPARTMENT OF AGRICULTURE  
SOIL CONSERVATION SERVICE

H. Lewis Hall 11/69

R. A. STALTER 11-69

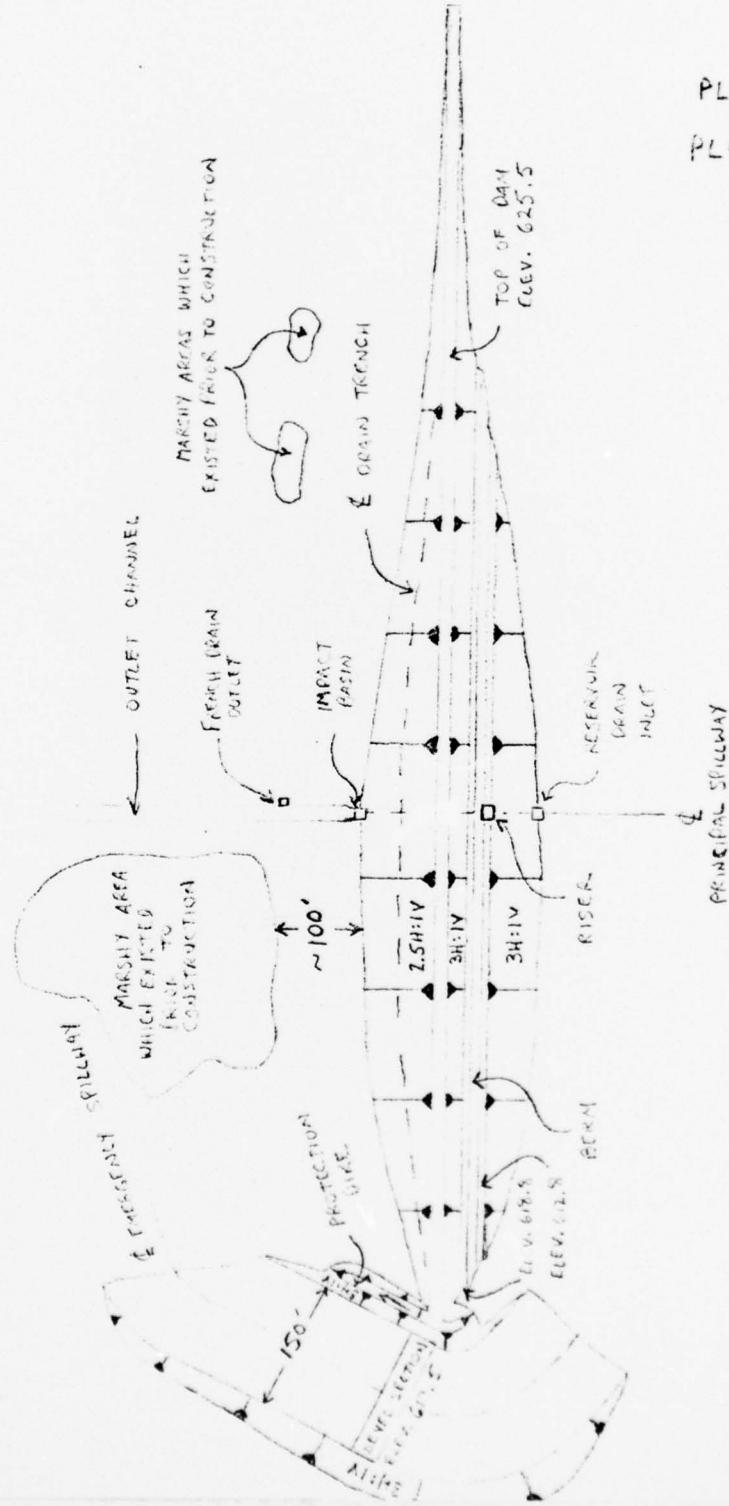
PA-431-P



**O'BRIEN & GERE  
ENGINEERS, INC.**

SUBJECT	ROBERT G. STRUBLE DAM	SHEET	BY	DATE	JOB NO
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PLATE 4  
PLAN VIEW



SUBJECT

ROBERT G. STRUBLE DAM

SHEET

BY  
PRB

DATE

JOB NO

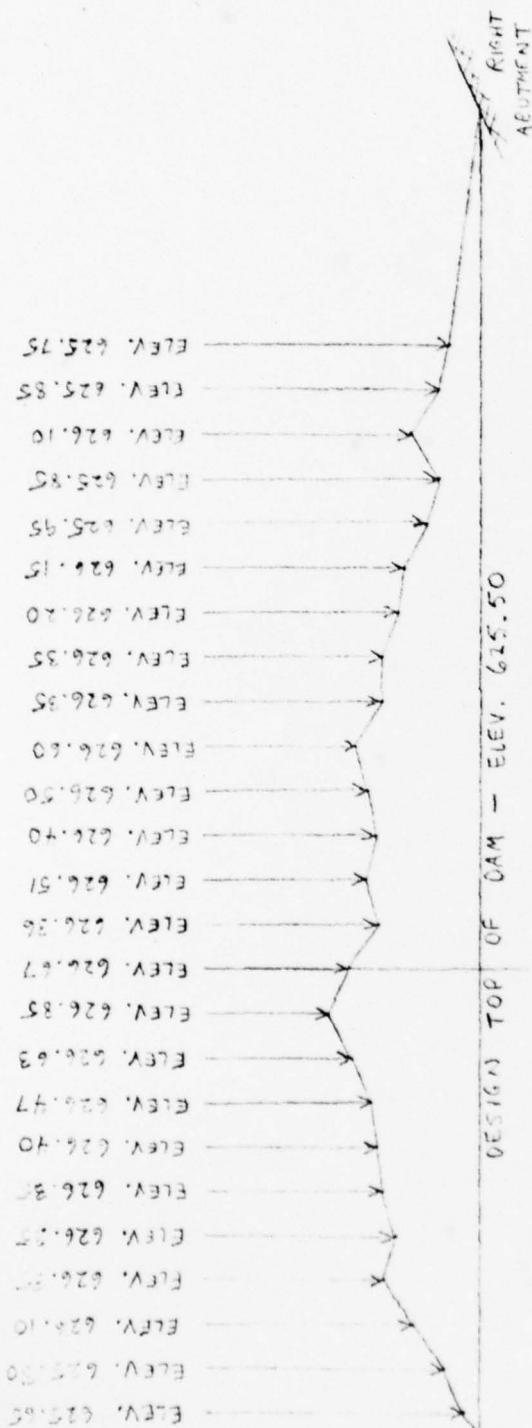


PLATE 5

TOP OF DAM PROFILE

HOR. SCALE : 1" = 200'  
VERT. SCALE : 1" = 1.6'

PRINCIPAL SPILLWAY  
CREST ELEV. 615.80

APPENDIX

F

Site Geology

## SITE GEOLOGY

### ROBERT G. STRUBLE DAM

Robert G. Struble Dam is located in the Uplands section of the Piedmont physiographic province. Bedrock at the site is a Precambrian quartz monzonite of igneous origin. This rock unit has differentially weathered in-situ such that its character varies throughout the site from a soil-like material exhibiting cohesion and/or granulation to a broken and fractured hard rock mass. The results of exploratory bore-holes at the site indicate that the thickness of the weathered material (which exhibits remnant rock structure) varies from 2 to 31 feet. No faults or major structural defects are noted in the vicinity of the dam or reservoir.

